틆	$F_y = 50 \text{ ksi}$
æ	$F_y = 50 \text{ ksi}$ $F_u = 65 \text{ ksi}$
<u>e</u>	$F_{V} = 36 \text{ ksi}$

Table 10-1 (continued) All-Bolted Double-Angle Connections

₹	$F_{tr} = 58 \text{ KSI}$	Bolt and Angle Available Strength, kips											
	4 Rows	D-10	Thread Cond.	Hole Type	Angle Thickness, in.								
		Group			1/4		5/16		3/8		1/2		
W24, 21, 18, 16		шопр	oona.	· ypc	ASD	LRFD	ASD	LRFD	ASD	LRFD	ASD	LRFD	
			N.	STD	67.1	101	83.9	126	95.5	143	95.5	143	
		- -	×	STD	67.1	101	83.9	126	101	151	120	180	
			SC Class A	STD	50.6	75.9	50.6	75.9	50.6	75.9	50.6	75.9	
		Group		ovs	43.1	64.5	43.1	64.5	43.1	64.5	43.1	64.5	
	Varian	A		SSLT	50.6	75.9	50.6	75.9	50.6	75.9	50.6	75.9	
	\$40°\$1 \$7° \$1°		SC Class B	STD	67.1	101	83.9	126	84.4	127	84.4	127	
	1 3 3			ovs	65.3	97.9	71.9	108	71.9	108	71.9	108	
				SSLT	65.8	98.7	82.2	123	84.4	127	84.4	127	
.51	1	Group	N X	STD	67.1	101	83.9	126	101	151	120	180	
3	12			STD	67.1	101	83.9	126	101	151	134	201	
	11 7		sc	STD	63.3	94.9	63.3	94.9	63.3	94.9	63.3	94.9	
				ovs	53.9	80.7	53.9	80.7	53.9	80.7	53.9	80.7	
	В	Class A	SSLT	63.3	94.9	63.3	94.9	63.3	94.9	63.3	94.9		
			SC Class B	STD	67.1	101	83.9	126	101	151	105	158	
				ovs	65.3	97.9	81.6	122	89.9	134	89.9	134	
				SSLT	65.8	98.7	82.2	123	98.7	148	105	158	

Hole Type Lev, in.		STD				ovs				SSLT			
		L _{ot} , in.											
		11/2		13/4		11/2		13/4		11/2		13/4	
		ASD	LRFD	ASD	LRFD	ASD	LRFD	ASD	LRFD	ASD	LRFD	ASD	LRFD
	11/4	167	250	175	262	156	234	164	246	164	245	172	257
	13/8	169	254	177	266	158	238	167	250	166	249	174	261
Coped at Top	11/2	171	257	180	269	161	241	169	254	168	253	177	265
Flange Only	15/s	174	261	182	273	163	245	171	257	171	256	179	268
	2	181	272	189	284	171	256	179	268	178	267	186	279
	3	201	301	209	313	190	285	198	297	198	296	206	309
	11/4	156	234	156	234	146	219	146	219	156	234	156	234
	13/8	161	241	161	241	151	227	151	227	161	241	161	241
Coped at Both	11/2	166	249	166	249	156	234	156	234	166	249	166	249
Flanges	15/s	171	256	171	256	161	241	161	241	171	256	171	256
	2	181	272	185	278	171	256	176	263	178	267	185	278
	3	201	301	209	313	190	285	198	297	198	296	206	309
Uncoped		234	351	234	351	234	351	234	351	234	351	234	351

Support Available Strength per Inch Thickness, kips/in.

STD - Standard holes OVS - Oversized holes

N - Threads included X — Threads excluded

SSLT - Short-slotted holes transverse to direction of load

SC - Slip critical

Hole ASD LRFD Type STDV OVS/ 468 702 SSLT

 Tabulated values include ¹/₄-in, reduction in end distance, L_{ab}, to account for possible underrun in beam length.

Note: Stip-critical bolt values assume no more than one filler has been provided or bolts have been added to distribute loads in the fillers.

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Dr. Ashad Ullah Qureshi

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considers the combined effects of prying action due to flexible end plates the formation of yield lines in the end plates and failures due to punching shear and beam section failure The model is calibrated and validated using experimental data from a test program The design model constitutes a relatively simple method for predicting the serviceability and ultimate moment capacities for the particular type of bolted moment end plate connection described herein *Modern Steel Construction*, 2007 *Tubular Structures XIV* Leroy Gardner, 2012-08-24 Tubular Structures XIV contains the latest scientific and engineering developments in the field of tubular steel structures as presented at the 14th International Symposium on Tubular Structures ISTS14 Imperial College London UK 12 14 September 2012 The International Symposium on Tubular Structures ISTS has a long standing reputation for b

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