

Beam		$F_y = 50$ ksi $F_u = 65$ ksi		Table 10-1 (continued) All-Bolted Double-Angle Connections										$\frac{3}{4}$ -in. Bolts			
Angle		$F_y = 36$ ksi $F_u = 58$ ksi		Bolt and Angle Available Strength, kips													
4 Rows W24, 21, 18, 16				Bolt Group	Thread Cond.	Hole Type	Angle Thickness, in.										
							$\frac{1}{4}$		$\frac{5}{16}$		$\frac{3}{8}$		$\frac{1}{2}$				
				ASD	LRFD	ASD	LRFD	ASD	LRFD	ASD	LRFD	ASD	LRFD				
				Group A	N	STD	67.1	101	83.9	126	95.5	143	95.5	143			
					X	STD	67.1	101	83.9	126	101	151	120	180			
					SC Class A	STD	50.6	75.9	50.6	75.9	50.6	75.9	50.6	75.9			
						OVS	43.1	64.5	43.1	64.5	43.1	64.5	43.1	64.5			
						SSLT	50.6	75.9	50.6	75.9	50.6	75.9	50.6	75.9			
					SC Class B	STD	67.1	101	83.9	126	84.4	127	84.4	127			
						OVS	65.3	97.9	71.9	108	71.9	108	71.9	108			
						SSLT	65.8	98.7	82.2	123	84.4	127	84.4	127			
				Group B	N	STD	67.1	101	83.9	126	101	151	120	180			
					X	STD	67.1	101	83.9	126	101	151	134	201			
					SC Class A	STD	63.3	94.9	63.3	94.9	63.3	94.9	63.3	94.9			
						OVS	53.9	80.7	53.9	80.7	53.9	80.7	53.9	80.7			
						SSLT	63.3	94.9	63.3	94.9	63.3	94.9	63.3	94.9			
					SC Class B	STD	67.1	101	83.9	126	101	151	105	158			
						OVS	65.3	97.9	81.6	122	89.9	134	89.9	134			
						SSLT	65.8	98.7	82.2	123	98.7	148	105	158			
Beam Web Available Strength per Inch Thickness, kips/in.																	
Hole Type		STD				OVS				SSLT							
		$L_{eh}^*,$ in.															
$L_{ev},$ in.		$1\frac{1}{2}$		$1\frac{3}{4}$		$1\frac{1}{2}$		$1\frac{3}{4}$		$1\frac{1}{2}$		$1\frac{3}{4}$					
		ASD	LRFD	ASD	LRFD	ASD	LRFD	ASD	LRFD	ASD	LRFD	ASD	LRFD				
Coped at Top Flange Only	$1\frac{1}{4}$	167	250	175	262	156	234	164	246	164	245	172	257				
	$1\frac{3}{8}$	169	254	177	266	158	238	167	250	166	249	174	261				
	$1\frac{1}{2}$	171	257	180	269	161	241	169	254	168	253	177	265				
	$1\frac{5}{8}$	174	261	182	273	163	245	171	257	171	256	179	268				
	2	181	272	189	284	171	256	179	268	178	267	186	279				
Coped at Both Flanges	3	201	301	209	313	190	285	198	297	198	296	206	309				
	$1\frac{1}{4}$	156	234	156	234	146	219	146	219	156	234	156	234				
	$1\frac{3}{8}$	161	241	161	241	151	227	151	227	161	241	161	241				
	$1\frac{1}{2}$	166	249	166	249	156	234	156	234	166	249	166	249				
	$1\frac{5}{8}$	171	256	171	256	161	241	161	241	171	256	171	256				
Uncoped	2	181	272	185	278	171	256	176	263	178	267	185	278				
	3	201	301	209	313	190	285	198	297	198	296	206	309				
		234	351	234	351	234	351	234	351	234	351	234	351				
Support Available Strength per Inch Thickness, kips/in.		Notes: STD = Standard holes OVS = Oversized holes SSLT = Short-slotted holes transverse to direction of load N = Threads included X = Threads excluded SC = Slip critical															
Hole Type	ASD	LRFD	* Tabulated values include $\frac{1}{4}$ -in. reduction in end distance, L_{eh} , to account for possible under-run in beam length.														
STD/OVS/SSLT	468	702	Note: Slip-critical bolt values assume no more than one filler has been provided or bolts have been added to distribute loads in the fillers.														

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Design of Steel Structures Elias G. Abu-Saba, 2012-12-06 This book is intended for classroom teaching in architectural and civil engineering at the graduate and undergraduate levels Although it has been developed from lecture notes given in structural steel design it can be useful to practicing engineers Many of the examples presented in this book are drawn from the field of design of structures Design of Steel Structures can be used for one or two semesters of three hours each on the undergraduate level For a two semester curriculum Chapters 1 through 8 can be used during the first semester Heavy emphasis should be placed on Chapters 1 through 5 giving the student a brief exposure to the consideration of wind and earthquakes in the design of buildings With the new federal requirements vis a vis wind and earthquake hazards it is beneficial to the student to have some understanding of the underlying concepts in this field In addition to the class lectures the instructor should require the student to submit a term project that includes the complete structural design of a multi story building using standard design procedures as specified by AISC Specifications Thus the use of the AISC Steel

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Steel Connection Design by Inelastic Analysis IDEA StatiCa, Mustafa Mahamid, Mark Denavit, Ali Nassiri, Halil Sezen, Martin Vild, 2024-10-15 Comprehensive resource on the finite element method in structural steel connection design through verification with AISC 360 provisions Steel Connection Design by Inelastic Analysis covers the use of the finite element method in structural steel connection design Verification with AISC 360 provisions is presented focusing on the Component Based Finite Element Method CBFEM a novel approach that provides the global behavior and verification

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