Table 6-2 (continued)

Available Strength for Members Subject to Axial, Shear, Flexural and Combined Forces

d Combined Forces
W-Shapes

W12

W12×				Shar	pe	W12×							
72		65		58		Ib/ft		72		65 ¹		58	
Pal De	ФcРa	P_{α}/Ω_{c}	ΦcPn	Palsie	ΦcPn			MaxIsh	Φ ₀ M _{mx}	May/Ob	Φ ₂ M _{mx}	May Do	$\Phi_{2}M_{nx}$
Available Compressive Strength, kips				Design		Available Flexural Strength, kip-ft							
ASD	LRFD	ASD	LRFD	ASD	LRFD			ASD	LRFD	ASD	LRFD	ASD	LRFD
632	949	572	859	509	765		0	269	405	237	356	216	324
606	911	549	825	479	720		6	269	405	237	356	216	324
597	898	540	812	469	705	_	7	269	405	237	356	216	324
587	883	531	798	457	687	4	8	269	405	237	356	216	324
576	866	521	783	445	668	8	9	269	405	237	356	215	323
564	847	510	766	431	647	gyration, ing	10	269	405	237	356	211	318
550	827	497	747	416	625	2 2	11	268	404	237	356	207	312
536	806	484	728	400	601	5 B	12	265	398	237	356	204	306
521 505	783	470 456	707 685	384	577		13	261	392	233	350 345	200	301
489	759 735	441	663	349	551 525	adius mis be	15	257 254	381	230	340	196	295 289
472	709	426	640	332	499	ti es	16	250	276	222	234	180	283
455	683	410 393	616	314	472 445		17	246 242	370	219	329	185	278 272
437 419	656 629	377	591 567	296 278	418	5 5	19	239	359	212	323 318	177	266
401	602	360	542	261	392	8 =	20	235	353	208	313	173	261
	547		492	227	341	2 3			342				
364 328	493	327 294	442	194	292	= =	22	228 220	331	201 194	302 291	166 158	249 238
292	440	262	394	165	249	with a	26	213	320	186	280	151	227
259	389	231	348	143	214		28	205	309	179	269	143	215
226	340	202	304	124	187	1,000 BOOK	30	198	297	172	259	135	203
199	299	178	267	109	164	-7.5	32	190	286	165	248	125	188
176	265	157	236	96.7	145	45.45	34	183	275	158	237	116	174
157	236	140	211	86.3	130	or unb	36	176	264	149	224	108	163
141	212	126	189	77.4	116		38	167	251	139	209	102	153
127	191	114	171	69.9	105	Effective	40	157	236	130	196	95.7	144
115	173	103	155			2	42	148	223	123	185	90.5	136
105	158	93.9	141			Side of the last	-64	140	211	116	175	85.8	129
96.2	145	85.9	129				46	133	200	110	166	81.6	123
88.3	133	78.9	119				48	127	191	105	158	77.8	117
81.4	122	72.7	109	-			50	121	182	100	150	74.3	112

Properties

A	vailable St	trength in	Tensile Yi	elding, kip	4			
PalOz	4.P.	Palica.	.Pa	Palitie	4.P.			
632	950	572	860	509	765			
Available Strength in Tensile Rupture $(A_{\mu} = 0.75A_{\mu})$, kips								
Paltite	♦ +P _m	Palitie	♦ePa	Palitie	ΦeP _m			
514	770	465	697	416	624			
Available Strength in Shear, kips								
Variou.	4.V_	ValCo.	. V.	Value.	. V.			
106	159	94.4	142	87.8	132			
Available Strength in Flexure about Y-Y Axis, kip-ft								
Mayellille	Φ2.May	Maylin	ф _b .M _{my}	Maylin	♦ 0.May			
123	185	107	161	81.1	122			

 $F_y = 50 \text{ ksi}$ $F_u = 65 \text{ ksi}$

	Limit	ing Unbra	ced Lengt	hs, ft		
Lp	Lr	Lp	E.	Lp	Le	
10.7	37.5	11.9	35.1	8.87	29.8	
		Area	, im.2			
21	.1	19	1.1	17.0		
	No.	forment of	Inertia, in	4		
f _a	Ly	Ew .	E _p	I.	I.	
597	195	533	174	475	107	
		Fpn	im.			
3.0	04	3.0	02	2.51		
		Fai	(r _r			
10.0	75	1.7	75	2.10		

[†] Shape exceeds compact limit for flexure with $F_y = 50$ kai. Note: Heavy line indicates L_c/r equal to or greater than 200.

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James R. Harris

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Steel Buildings Stanley W. Crawley, Robert M. Dillon, 1993 This volume presents the general principles of structural analysis and their application to the design of low and intermediate height building frames The text is accompanied by software for the analysis of axial forces displacement and the bending moment and the determination of shear

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Design of Steel Structures Elias G. Abu-Saba,2012-12-06 This book is intended for classroom teaching in architectural and civil engineering at the graduate and undergraduate levels Although it has been developed from lecture notes given in structural steel design it can be useful to practicing engineers Many of the examples presented in this book are drawn from the field of design of structures Design of Steel Structures can be used for one or two semesters of three hours each on the undergraduate level For a two semester curriculum Chapters 1 through 8 can be used during the first semester Heavy

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CMU walls concrete and wood walls connections detailing whether it is a moment or shear connection existing roof joi web and chord reinforcement and roof trusses section details	ists

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